scientific reports



OPEN

Bedload transport in rivers, size matters but so does shape

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Bedload transport modelling in rivers takes into account the size and density of pebbles to estimate particle mobility, but does not formally consider particle shape. To address this issue and to compare the relative roles of the density and shape of particles, we performed original sediment transport experiments in an annular flume using molded artificial pebbles equipped with a radio frequency identification tracking system. The particles were designed with four distinct shapes and four different densities while having the same volume, and their speeds and distances traveled under constant hydraulic conditions were analyzed. The results show that particle shape has more influence than particle density on the resting time between particle displacement and the mean traveling distance. For all densities investigated, the particle shape systematically induced differences in travel distance that were strongly correlated (R² = 0.94) with the Sneed and Folks shape index. Such shape influences, although often mentioned, are here quantified for the first time, demonstrating why and how they can be included in bedload transport models.

Sediment transport is a key process in fluvial geomorphology, being important for sustainable management of navigable channels, designing engineering projects, predicting morphological changes and associated hydraulic risks, interpreting sedimentary archives and restoring rivers¹. It involves three phases of particle mobility: (1) entrainment²⁻⁶; (2) motion⁷⁻⁹; and (3) deposition^{10,11}. Sediment transport at the particle scale is a stochastic phenomenon^{7-9,12-14}, which mostly arises from the complex interactions between particle collisions and highly variable friction, drag, and lift forces due to fluid turbulence. Thus, for practical considerations, empirically calibrated sediment transport functions widely use the Shields stress number (τ^*) to quantify the balance of the forces exerted on the channel bed particles, and the critical Shields number (τ^*), which is the threshold value necessary to set particles in motion, to determine the moments at which drag forces exceed stabilizing forces ($\tau^* > \tau^*_c$) and particles can be entrained¹⁵⁻²¹. Such approaches have been used to estimate particle stabilizing forces from median pebble size and submerged density¹⁶. At the river reach scale, sediment transport estimates generally encapsulate a relation depending on the Shields stress, and therefore also include the median grain size^{20,22-27} of the transported sediment.

Published bedload transport datasets from rivers with similar flow conditions, morphologies, and median grain sizes, may show different transport rates, with large variations in the threshold for particles motion²⁸, variations that can be up to tenfold²⁹ around the mean empirical Shields curve^{30–32}. To explain such dispersion, many studies have focused on the role of mixed grain size, hiding effects^{33–36}, macro-roughness, channel steepness, or bed roughness relative to channel depth³⁷. However, fewer studies have qualitatively studied the influence of pebble shape on bedload transport through its effect on angularity^{38,39}, pebble imbrication^{34,35,40}, or bed roughness^{34,40,41} (i.e. impact of the D/K ratio, where D is the diameter of the particles to be moved and K is the bed-particle diameter). In environments with smooth-beds (D>K) and during low to moderate flood events, coarse particles of spherical or ellipsoid shape were observed⁴² to be more likely to experience entrainment and transport than flatter shapes. Conversely, in rough-bed rivers (D<K), Demir and Walsh¹ found that displacement of flatter shapes (i.e. discs and blades) seems to be promoted. Overall, selective shape entrainment and travel length both decrease as flood magnitude increases and/or particle size decreases⁴³. Whereas these previous studies have emphasized that robust deterministic expression of initial motion should encapsulate the role of particle shape and bed roughness in particle motion modelling^{38,39,44,45}, the scarcity of field and experimental data has prevented a quantitative account of this role.

To partially fill this gap, we designed a parametric study based on experiments run in an annular flume (see the method section) in which the displacements (encapsulating onset motion, travel length and rest periods) of

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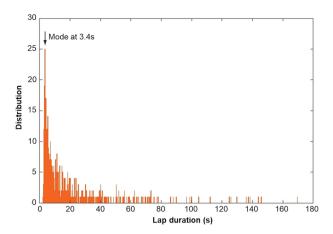


Figure 1. Example of the distribution of lap durations (shape = disc; density = 2.4 g cm^{-3}).

artificial pebbles of various shapes and densities were tracked for several hours. Particle shape has been quantified by many different parametrizations^{46–52} expressing angularity, surface roughness, or departure from sphericity. As the latter directly impacts on inertial moments and pivoting angle, we investigated the influence of shape in terms of the departure from sphericity, examining various ellipsoid particle shapes (from plate to blade types).

Results

The number of revolutions recorded for the monitored particles ranged between 439 laps for an elongated blade and 2270 laps for a sphere during the same period, ensuring that the lap duration measurements were extracted from large samples. Although the lap durations within the annular flume displayed large variations (from 3 s up to a few minutes; see example in Fig. 1) over the total run duration, the cumulative travel distances of the particles (Fig. 2) displayed a fairly constant slope that permitted the average traveling velocities of the different artificial pebbles to be defined.

The slight increases observed in the slopes of the cumulative distance curves over time for all shapes and densities reflect the progressive augmentation of the particles' velocities caused by a decrease in the mixing load due to abrasion (relative mass loss of 1.2% per kilometer traveled). As this effect was minor and affected similarly all tagged particles, it was concluded that it had a little impact on the first-order estimates and results of the experiments.

Both the particle shape and density exhibited significant differences in the cumulative travel length (Fig. 2). The spherical particles traveled the farthest and fastest (mean velocities ranging from 0.44 to 0.60 ms^{-1}), with the mean virtual velocity displaying an inverse relationship with density (Fig. 3). The compact blade-shaped particles were the second fastest, exhibiting mean velocities ranging between 0.25 and 0.44 ms^{-1} , again displaying an inverse relationship with density, although to a lesser extent than that of the spherical particles. In contrast, the mean virtual velocities of the disc- and elongated blade-shaped particles were minimally influenced by density: the mean velocities were clustered within a narrow range from 0.14 to 0.17 ms^{-1} and 0.19 to 0.21 ms^{-1} respectively. Within the density classes, the distances traveled by particles, clearly showed a high variability in relation to their shapes (Figs. 2B and 3A). The experiments clearly indicate that the variability in velocity associated with pebble shape is substantially higher than that associated with particle density ($\sim 100\%$ compared with $\sim 30\%$).

To explore the influence of particle shape on mobility in a more quantitative way, we used the sphericity index, Ψ_p (1), of Sneed and Folks (1958):

$$\Psi_p = \sqrt[3]{\frac{S^2}{LI}} \tag{1}$$

where *L*, *I*, and *S* are the longest, intermediate, and shortest axes of the pebbles.

The sphericity index Ψ_p shows a remarkable positive relationship with the mean traveling velocity (Fig. 3B). Moreover, the mean velocities increased from 0.52 to 0.85 ms⁻¹ for decreasing densities from 2.6 to 2.0 g cm⁻³. These results suggest that it is possible to estimate differences in the mean virtual velocities and mobilities of particles according to their sphericity.

The lap-scaled average travel velocities integrate the duration of motion phases and the resting periods between one phase and the following one. The pebble shape and density can influence the rest and the motion phase differently. The lap duration distributions are characterized by a first peak at around 3 s in all experiments (Fig. 1), which corresponds to a revolution speed of $\sim 1.2~\text{ms}^{-1}$. For experimental conditions similar to those used in this study, high speed camera viewing ⁵³ previously indicated a mean hop velocity of $1.2\pm0.2~\text{ms}^{-1}$ for pebbles in an annular flume. This modal lap duration of $\sim 3~\text{s}$ therefore represents a continuous succession of hops over a full lap, without any resting time. These modal values decrease slightly with increasing density (Fig. 4A), as expected from the larger inertial effect after the pebble is set in motion. More importantly, they are almost independent of the pebble shape, as was also observed in a straight-flume study ⁵⁴. This implies that the influence of

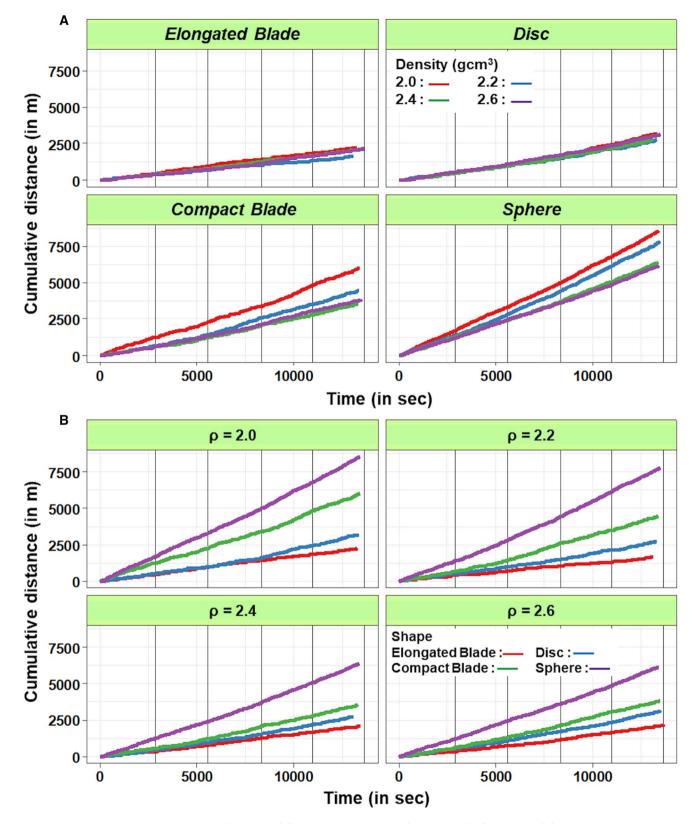


Figure 2. Cumulative travel distances over time according to particle shape (A) and density (B).

shape on the mean traveled distance is mostly caused by its influence on the resting time between movements, i.e. on the immobilization conditions and on the threshold for setting pebbles in motion. To illustrate this inference, a simple calculation of the mean resting time fraction, or immobility ratio (I_r) , can be estimated through

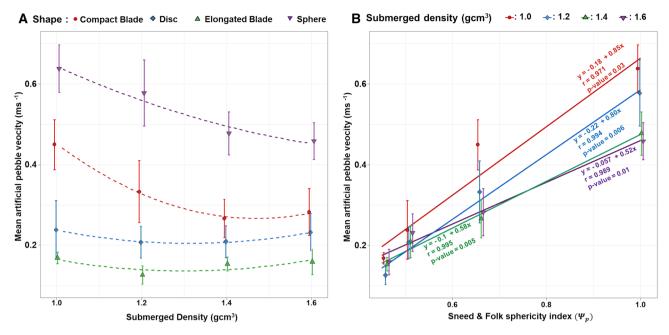


Figure 3. Mean velocity as function of density (**A**) and Sneed and Folk spherical index (**B**).

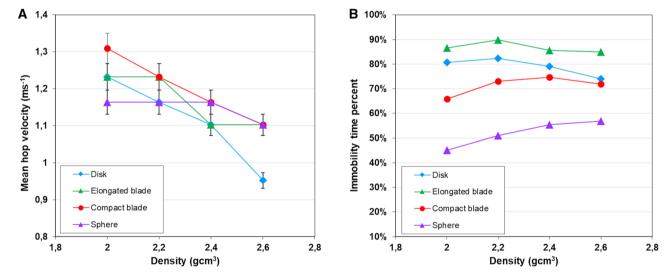


Figure 4. (A) Mean hop velocity and (B) time fraction of immobility of the 16 different artificial pebbles.

$$I_r = \frac{T - N_l t_m}{T},\tag{2}$$

with T being the total duration of the runs, N_l the number of achieved flume revolutions during T, and t_m the modal lap duration (first mode on the distribution of Fig. 1) corresponding to a continuous succession of hops over a full lap.

Except for spherical pebbles that display a slight increase, the immobility ratio (Fig. 4B) is only weakly or not at all affected by particle density. In contrast, the shape of a pebble deeply impacts on its mobility, with the immobility ratio raging from $\sim 50\%$ for the spherical shapes up to $\geq 85\%$ for the elongated blades.

Discussion

The greater velocity of the spherical and compact-blade-shaped particles that was found in this research compared to the elongated-blade and disc-shaped particles is in good agreement with the literature^{1,39,55}, given that the flatness of the flume bottom constitutes a low roughness bed surface, despite clustering of temporary resting pebbles. As most pebble lithologies in rivers show a density close to 2.7 g cm⁻³, which is similar to the highest value used in this study, we expect their mean velocities to be more strongly influenced by their shape than by

their density. On a quantitative basis, this supports the claimed need to include a particle shape parameter in the sediment transport equation ^{34,41,56}.

To do this, we focus on the conditions for setting a particle in motion, starting from the assumption that pebble shape has a major influence on virtual velocity through resting periods. Following Komar and Li's (1986)⁴¹ description, balancing of the moments of tractive and resisting forces for the critical stress yields:

$$\tau_c \propto \frac{l_W \Delta \rho g S I L}{l_D A_a}$$
(3)

where A_a is the apparent section exposed to the flow, and l_D and l_w the respective moment arms of the drag force and submerged weight respectively. Assuming that pebbles tend to lie with their S-axis vertically oriented, the moment arms of the drag force l_D approximately scales with the S-axis. As a pebble can orient either longitudinally or transversally, we use the intermediate variable \sqrt{LI} to account for the apparent section exposed to the flow $(A_a \propto S\sqrt{LI})$ and the moment arm of the submerged weight l_w . Therefore:

$$\tau_c \cong k \frac{\sqrt{LI} \Delta \rho g \sqrt[3]{(SIL)^2} \tilde{D}}{S^2 \sqrt{LI}} = k \sqrt[3]{\frac{(IL)^2}{S^4}} \Delta \rho g \tilde{D} = k \frac{1}{\Psi_p^2} \Delta \rho g \tilde{D}$$
 (4)

where k is a function of the particles' Reynold number considered as a constant, $\tilde{D} = \sqrt[3]{SIL}$, the mean pebble size, and $\Psi_P = \sqrt[3]{\frac{S^2}{IL}}$, the Sneed and Folk's index. Here, $\frac{1}{\Psi_P^2}$ corresponds more or less to the term $\tan \phi$ in Komar and Li (1986): when particle flatness increases (i.e. Ψ_P decreases), the pivoting angle increases and mobility is reduced. Suppressing the unknown k, the threshold can be expressed as:

$$\tau_c \cong \left(\frac{\Delta \rho}{\Delta \rho_{ref}}\right) \left(\frac{\Psi_{P_{ref}}}{\Psi_{P}}\right)^2 \tau_{c_{ref}} \tag{5}$$

where $\tau_{c_{ref}}$ is the critical Shields stress of a reference pebble of similar size.

The non-dimensional critical threshold is expressed as:

$$\tau_c^* \cong \frac{\tau_c}{\Delta \rho g \tilde{D}} = \frac{k}{\Psi_p^2} = \left(\frac{\Psi_{P_{ref}}}{\Psi_P}\right)^2 \tau_{c_{ref}}^* \tag{6}$$

where $\tau_{c_{ref}}^*$ is the critical Shields stress of a reference pebble of similar size.

When the mean travel velocity of particles is expressed as a function of the critical stress τ_o an inverse correlation between the two variables results (Fig. 5): both an increase of and a decrease of sphericity decrease the ratio of tractive over resistive moments and favor particle immobility.

Most bedload transport capacity formulae are functions of the excess Shields stress and follow two general forms: (1) $\Phi = K(\tau^* - \tau_c^*)^{\alpha}$, and (2) $W^* = (\tau/\tau_c)^{\alpha}$, where Φ and W^* are two distinct non-dimensional expressions of the bedload transport rate, and α and K two constant terms⁵⁷. To account for the role of pebble shape in a transport capacity relationship, one could introduce the modified expression of critical shear stress (Eq. 5) into the formula, or the critical Shields stress (Eq. 6) that includes the Sneed and Folk Index.

To explore this hypothesis, we built on the fractional transport rate model developed for transport of a mixture of grain sizes (e.g. Parker et al., 1982⁵⁸). This choice was motivated by the fact that such a relation already proposes a similarity collapse for heterogeneous sediments, which is the case in our experiments with particles of variable shapes and densities mixed with a natural pebble load. We arbitrarily considered Wilcock and Crowe's (2003)⁵⁹ relation for fractional transport rate, in which the form of the similarity collapse is:

$$W_i^* = 14 \left(1 - \frac{0.894}{\phi^{0.5}} \right)^{4.5}$$
 when $\phi = \frac{\tau}{\tau_{ci}} \ge 1.35$ (7)

where τ is the bed shear stress, τ_{ci} the critical shear stress for incipient motion of a specific pebble i (more exactly it corresponds to the minimum shear stress required to achieve a small reference transport rate of $W_i^* = 0.002^{58}$), and W_i^* the dimensionless transport rate $W_i^* = \frac{Rgq_{bi}}{F_i\left(\frac{\tau}{\rho}\right)^{3/2}}$, with $R_i = \frac{\Delta\rho_i}{\rho}$ being the ratio of the submerged sedi-

ment (of type i) density to water density, g being gravity, $q_{\rm bi}$ the volumetric transport rate per unit width of the particle of type i (i.e. of similar shape, size, and density), and F_i the proportion of the pebble type being of the class i.

Following our simplified analysis of the balance of force momentum, we defined the critical (or reference) shear stress as a function (Eq. 8) of the mean characteristics of the transported sediment load (i.e. mean gravel size D_m , mean shape factor p_m , and mean density $\Delta \rho_m$) according to:

$$\tau_{ci} = \left(\frac{\Delta \rho_i}{\Delta \rho_m}\right) \left(\frac{\Psi_{Pm}}{\Psi_{Pi}}\right)^2 \tau_{cm} \tag{8}$$

with $\tau_{\rm cm}$ being the critical shear stress for the mean gravel load. Here, $\tau_{\rm cm} = \Delta \rho_m g D_m \tau_c^* \cong 28$ Pa considering that $\Delta \rho = 2600$ kg m⁻³, $D_{\rm m} \approx 5$ cm for the mean gravel diameter of the 65 kg of limestone pebbles, and $\tau_c^* \cong 0.036^{59}$.

Within the flume, provided that not all particles are in full motion, the conditions of alluvial rivers prevail, i.e. the sediment flux q_{si} is equated by the transport capacity q_{bi} . In our experiments, the mass sediment flux per unit width of the pebble class i can be expressed from the mean traveling velocity through: $q_{si} = \frac{F_i M}{A} V_{gi}$, with

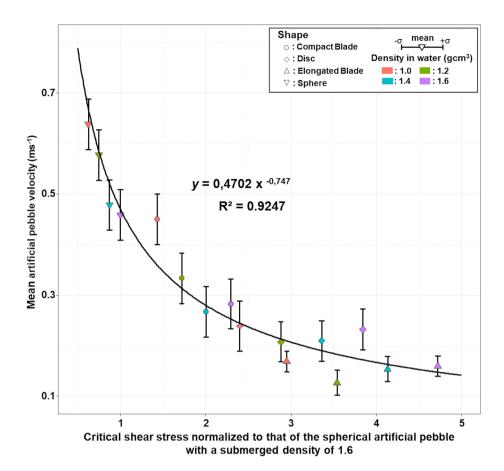


Figure 5. Mean velocity of the 16 artificial pebbles vs. their critical shear stress normalized to that of the spherical artificial pebble with a submerged density of $1.6 \left(= \frac{\tau_c}{\tau_{c_{ref}}} = \left(\frac{\Delta_{\rho}}{1.6} \right) \left(\frac{1}{\Psi_P} \right)^2 \right)$.

A being the surface of the flume bottom, M the mass of sediment introduced into the flume, and V_{gi} the mean displacement velocity of particles of type i. It follows that a virtual mean velocity can be derived for particle i from the above fractional transport rate equation:

$$V_{gi} = \frac{AM\rho_{si}}{R_{i}g} \left(\frac{\tau}{\rho}\right)^{3/2} W_{i}^{*} \left(\frac{\tau}{\tau_{ci}}\right)$$
(9)

with τ_{ci} derived from Eq. (8) and a mean shape factor $\Psi_{Pm} = 0.7 \pm 0.08$, which corresponds to the 65 kg of rounded limestone pebbles, most of which have a shape close to that of a compact ellipsoid, and with which our tracked artificial pebbles were mixed.

The virtual velocities derived from the bedload transport relation show a well-defined correlation with the measured virtual velocities (Fig. 6). However, the slope of the correlation line is larger than unity, and our modified version of the bedload transport tends to underestimate the observed transport for the densest elongated-blade or disk-shaped pebbles. Despite these slight discrepancies from the observations, these results suggest that the role of pebble shape on bedload transport can be predicted, and that the inclusion of pebble shape characteristics in the modelling of bedload transport offer much promise for improving bedload transport predictions.

In terms of sediment dynamics, pebbles travel in the flume following an alternating pattern of resting and motion periods, as generally observed in a natural stream⁸. We therefore consider that our experiments succeeded in capturing the first order behavior of the bedload, and that the introduction of a shape factor into the critical Shield stress and bedload transport models might be transposed to rivers. However, the experimental conditions are slightly different from those of natural rivers, in particular the use of a monodispersed sediment load and a low-roughness bottom. Additional experiments exploring distinct bottom conditions, grain size distributions or shapes, and using straight channels are probably necessary to strengthen our initial results and resolve the slight discrepancies between the model and observations. Experiments using pebbles with a unique and defined type of shape, instead of a single particle mixed with a large population of pebbles of distinct shapes, would allow, for example, the examination of fluvial transport of predominantly flattened particles such as those resulting from the erosion of schist- or shale-rich lithologies. Similarly, our experiments were conducted with relatively well-rounded limestone pebbles, whereas the upstream reaches of mountain river networks tend to be characterized by particles with more irregular or angular shapes. Although some part work⁶⁰ has suggested no relation between pebble angularity and the resistance to initial movement of the particle, we can hypothesize that

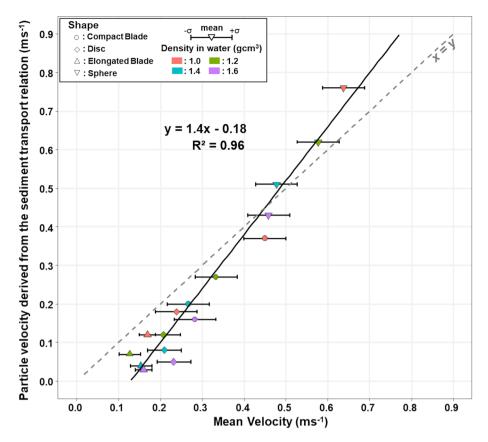


Figure 6. Comparison between the mean measured velocities of particles of various shapes and densities and the theoretical particle velocity derived from a fractional transport rate relation adapted from Wilcock and Crowe's (2003) relation^{59.}

significant particle angularity might promote particle imbrication and reduce bedload mobility and transport. Future experiments could thus be designed to explore the influence of angularity on bedload transport capacity. Our experiments presented in this paper, should help to derive a more universal relationship applicable to natural rivers with heterogeneous mixtures of sizes and shapes comprising the bedload. The present study can therefore be considered as a preliminary step towards addressing the role of particle shape in bedload transport. All existing field data sets do not comprise information about grains shape. In order to fully explore these properties, a protocol needs also to be defined for collecting this information in future field campaigns.

Conclusion

The experiments performed in this research, which are based on innovative tools (artificial pebbles of controlled density containing RFIDs) offer new perspectives for studying sediment transport mechanisms. The comparative analysis of the shape and density of particles on their mobility highlights the crucial influence of particle shape. Furthermore, it also indicates that the sphericity index (Ψ_P) of Sneed and Folks (1958)⁶¹, which correlates with mean velocity, is relevant for including shape parameters in sediment transport formulae. The method developed in this study can be reproduced to investigate how bed roughness (changing D/K ratio) and/or a tracer's grain-size can change the balance between the effects of shape and density on particle velocity. It allows the investigation of whether bed roughness promotes the transport of flat-shaped particles, as reported in the literature, and whether particle density can mitigate this effect. Repeating the experiments with smaller particle sizes (maintaining a constant D/K ratio) would also allow investigation of whether size mitigates the influence of shape and density on particle transport.

Methods

We designed four differently-shaped particle models within the grain-size class of 45–64 mm (5.5–6.0 Ψ-units), with all models having the same volume (i.e. 49.3 cm³) but exhibiting differences in the sphericity index⁶² (Fig. 7A; Table 1). After creating silicon molds (RTV 120) for these four models, we manufactured 16 artificial pebbles using a mixture of resin and corundum powder in variable proportions, creating pebbles of four different densities (2.0, 2.2, 2.4, and 2.6 g cm⁻³) for each mold shape⁶³. We equipped these artificial pebbles with transponders of Radio Frequency Identification, RFID, (model RI-TRP-WR2B of Texas Instrument, Dallas Texas USA, also known as PIT Tags) to monitor their displacements within an annular flume (Fig. 7B). A detection antenna located on the outside of the flume, along a lateral window, enabled tracking of the number of laps achieved by the RFID-equipped pebbles and the time for each revolution.

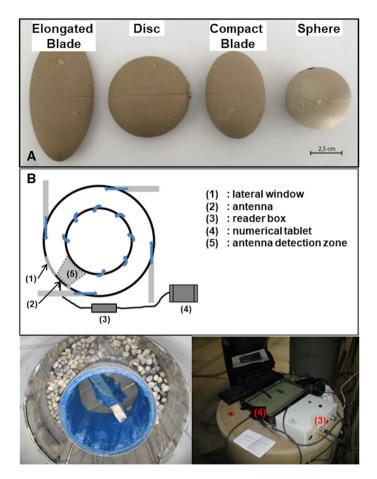


Figure 7. The four particle shapes investigated (**A**) and the annular flume equipped with the RFID system (**B**).

SHAPE	a-axis (mm) L	b-axis (mm) I	c-axis (mm) S	Vol. (cm ³)	Sphericity index (Sneed and Folks, 1958)
Compact Blade	68.1	46	30	49.3	0.66
Sphere	45.5	45.5	45.5	49.3	1
Disc	65	63	23	49.3	0.51
Elongated Blade	97.2	46.1	21	49.3	0.46

Table 1. Shape characteristics of the artificial particles tracked in the flume.

In an attempt to reproduce bedload transport conditions, these artificial pebbles were mixed with 65 kg of limestone pebbles of a similar grain-size (i.e. class 45-64 mm) and were run within an annular flume. The external diameter and width of the flume are 1.5 m and 0.3 m respectively, and its outer and inner heights are 1.5 m and 0.6 m respectively⁶³. The water circulation that allows the movement of the pebbles at the bottom of the flume is induced by 4 tangential injections at the level of the outer wall of the flume (at a height of 0.6 m). The water then flows over the inner edge of the flume to fall into a tank before being recirculated to the injections by a powerful pump. Pebbles, on the other hand, remain permanently at the bottom of the flume, and travel an average distance of 3.77 m per lap. A set of experiments were run following the designs of previous studies^{53,63,64} for which the sediment dynamics have been characterized⁶⁴, i.e. with a low roughness bottom and a monodispersed grain size distribution. During the experiments, the pump discharge sustaining the fluid injection into the flume was maintained at 240 m³h⁻¹, which for the introduced sediment mass corresponds⁶⁵ to a shear stress of $\tau = 135$ Pa at the base of the flume according to Euler theorem applied to the moments, a Shield stress of τ^* = 0.16, a mean transit velocity for pebbles of $\approx 0.4 \text{ ms}^{-1}$, and a sediment flux of $\sim 24 \text{ kg m}^{-1} \text{ s}^{-1}$. Under these conditions, high speed camera viewing⁵³ indicated that the pebbles were transported in the annular flume in a similar manner to that observed8 in rivers, with alternating transport phases with rolling and saltation, and resting times caused by temporary blockage and piling of particles.

Each experimental run lasted 45 min. To avoid superpositioning of radio-frequency signals and missed RFID transponder detections 63,66,67 , only the particles (n = 4) with the same density were simultaneously present in the flume, so limiting the number of transponders to four. A total of six runs were achieved for the densities of 2.6

and $2.4 \,\mathrm{g}$ cm⁻³, and five runs for the densities of $2.0 \,\mathrm{and}\, 2.2 \,\mathrm{g}$ cm⁻³. For each artificial pebble, the combined runs provide a long duration of almost 4 h and a large cumulative traveled distance, from which the mean traveled velocity (or virtual velocity as defined by Haschenburger and Church⁶⁸) can be computed and the distributions of the lap times estimated. Finally, in order to investigate the effects of the different shapes and densities on bedload transport, the virtual velocities and lap distributions of the 16 artificial pebbles were compared. The use of an annular flume enabled the acquisition of a relatively long time series compared with typical straight flume experiments⁶⁹ and the sampling of a population of practically uncensored particle trajectories, without the limitations induced by a limited detection window or flume length⁷⁰. This ensured that the ranges of traveled distances, under conditions of continuous movement, were well represented in the experiment. We also made sure that the duration of the experiments (45 min) was much longer than the maximum resting time recorded (~5 min). This allowed both avoiding time censorship effects on the distributions of the resting periods and lap times, and increasing the statistical significance of the distributions.

Received: 20 August 2020; Accepted: 14 December 2020

Published online: 12 January 2021

References

- 1. Demir, T. & Walsh, R. P. D. Shape and size characteristics of bedload transported during winter storm events in the Cwm Treweryn Stream, Brecon Beacons, South Wales. *Turk. J. Earth Sci.* 14, 105–121 (2005).
- 2. Mears, A. I. Flooding and sediment transport in a small alpine drainage basin in Colorado. Geology 7, 53 (1979).
- 3. Bradley, W. C. & Mears, A. I. Calculations of flows needed to transport coarse fraction of boulder creek alluvium at Boulder, Colorado. *Geol. Soc. Am. Bull.* **91**, 1057–1090 (1980).
- 4. Einstein, H. A. & El-Samni, E.-S.A. Hydrodynamic forces on a rough wall. Rev. Mod. Phys. 21, 520-524 (1949).
- 5. Cheng, E. D. H. & Clyde, C. G. Instantaneous hydrodynamic lift and drag forces on large roughness elements in turbulent open channel flow. In 3-1-3-20 (H. W. Shen, 1972).
- 6. Komar, P. D. & Li, Z. Applications of grain-pivoting and sliding analyses to selective entrapment of gravel and to flow-competence evaluations. *Sedimentology* **35**, 681–695 (1988).
- 7. Einstein, A. H. Bedload transport as a probability problem. (Colorado State University, 1937).
- 8. Habersack, H. M. Radio-tracking gravel particles in a large braided river in New Zealand: A field test of the stochastic theory of bed load transport proposed by Einstein. *Hydrol. Process.* 15, 377–391 (2001).
- 9. Olinde, L. & Johnson, J. Using RFID and accelerometer-embedded tracers to measure probabilities of bed load transport, step lengths, and rest times in a mountain stream. *Water Resour. Res.* **51**, 7572–7589 (2015).
- Hjulström, F. Studies of the Morphological Activity of Rivers as Illustrated by the River Fyris. Inaugural dissertation. (Almqvist & Wiksells, 1935).
- 11. Einstein, A. H. Bedload Transport as a Probability Problem (Colorado State University, Fort Collins, 1937).
- Ergenzinger, P. & Schmidt, K. H. Stochastic elements of bed load transport in a steppool mountain river. Hydrol. Mt. Reg. II Artif. Reserv. Water Slopes Int. Assoc. Hydrol. Sci. Publ. 194, 39–46 (1990).
- 13. Busskamp, R. The influence of channel steps on coarse bed load transport in mountain torrents: case study using the radio tracer technique 'PETSY'. In *Dynamics and geomorphology of mountain rivers* 129–139 (Springer, 1994).
- 14. Olinde, L. Displacement and Entrainment Behavior of Bedload Clasts in Mountain Streams (University of Texas, Austin, 2015).
- 15. Shields, A. Application of Similarity Principles and Turbulence Research to Bed-Load Movement (CalTech Library, Pasadena, 1936).
- 16. Meyer-Peter, E. & Müller, R. Formulas for Bed-Load transport. IAHSR 2nd Meeting Stockholm Appendix 2 (1948).
- 17. Engelund, F. & Hansen, E. A monograph on sediment transport in alluvial streams. Tech. Univ. Den. 0stervoldgade 10 Cph. K (1967).
- 18. Ackers, P. & White, W. R. Sediment transport: new approach and analysis. J. Hydraul. Div. 99, 2041-2060 (1973).
- 19. Parker, G. & Klingeman, P. C. On why gravel bed streams are paved. Water Resour. Res. 18, 1409-1423 (1982).
- 20. Recking, A., Frey, P., Paquier, A., Belleudy, P. & Champagne, J. Y. Feedback between bed load transport and flow resistance in gravel and cobble bed rivers: feedback between bed load and flow resistance. *Water Resour. Res.* 44, 1–21 (2008).
- 21. Piton, G. & Recking, A. The concept of travelling bedload and its consequences for bedload computation in mountain streams: how to account for allogenic supply in bedload transport equations?. *Earth Surf. Process. Landf.* https://doi.org/10.1002/esp.4105 (2017).
- 22. Rickenmann, D. hyperconcentrated flow and sediment transport at steep slopes. J. Hydraul. Eng. 117, 1419–1439 (1991).
- 23. Smart, G. & Jäggi, M. Sediment transport on steep slopes. Mitteilung. 64. Versuchsanstalt fur Wasserbau, Hydrologie und Glaziologie. (ETH Zurich, Zurich, 1983).
- 24. Parker, G., Klingeman, P. C. & McLean, D. G. Bedload and size distribution in paved gravel-bed streams. *J. Hydraul. Div.* 108, 544–571 (1982).
- 25. Wilcock, P. R. & Crowe, J. C. Surface-based transport model for mixed-size sediment. J. Hydraul. Eng. 129, 120–128 (2003).
- 26. van Rijn, L. C. Sediment transport, part I: bed load transport. J. Hydraul. Eng. 110, 1431-1456 (1984).
- 27. Recking, A. Theoretical development on the effects of changing flow hydraulics on incipient bed load motion. *Water Resour. Res.* 45, 1–16 (2009).
- 28. Frey, P. & Church, M. How river beds move. Science 325, 1509-1510 (2009).
- 29. Buffington, J. M. & Montgomery, D. R. A systematic analysis of eight decades of incipient motion studies, with special reference to gravel-bedded rivers. *Water Resour. Res.* 33, 1993–2029 (1997).
- Gessler, J. Chapter 7 Preprint of Paper Beginning and Ceasing of Sediment Motion (Colorado State University, Fort Collins, 1970).
 Miller, M. C., McCAVE, I. N. & Komar, P. D. Threshold of sediment motion under unidirectional currents. Sedimentology 24, 507–527 (1977).
- 32. Yalin, M. S. & da Silva, A. M. F. Fluvial Processes (IAHR, London, 2001).
- 33. Miller, R. L. & Byrne, R. J. The angle of repose for a single grain on a fixed rough bed. *Sedimentology* **6**, 303–314 (1966).
- 34. Li, Z. & Komar, P. D. Laboratory measurements of pivoting angles for applications to selective entrainment of gravel in a current. *Sedimentology* **33**, 413–423 (1986).
- 35. Kirchner, J. W., Dietrich, W. E., Iseya, F. & Ikeda, H. The variability of critical shear stress, friction angle, and grain protrusion in water-worked sediments. *Sedimentology* 37, 647–672 (1990).
- 36. Buffington, J. M., Dietrich, W. E. & Kirchner, J. W. Friction angle measurements on a naturally formed gravel streambed: implications for critical boundary shear stress. *Water Resour. Res.* 28, 411–425 (1992).
- 37. Lamb, M. P., Brun, F. & Fuller, B. M. Hydrodynamics of steep streams with planar coarse-grained beds: turbulence, flow resistance, and implications for sediment transport: HYDRODYNAMICS OF STEEP STREAMS. *Water Resour. Res.* 53, 2240–2263 (2017).
- 38. Miller, R. L. & Byrne, R. J. The angle of repose for a single grain on a fixed rough bed. Sedimentology 6, 303-314 (1966).

- 39. Carling, P. A., Kelsey, A. & Glaister, M. S. Effect of bed roughness, particle shape and orientation on initial motion criteria. In *Dynamics of Gravel-bed Rivers* 24–39 (eds Hey, R.D. Billi, P., Thorne, C.R. and Tacconi, P.) (1992).
- 40. Lane, E. W. & Carlson, E. J. Some observations on the effect of particle shape on the movement of coarse sediments. *Eos Trans. Am. Geophys. Union* **35**, 453–462 (1954).
- 41. Komar, P. D. & Li, Z. Pivoting analyses of the selective entrainment of sediments by shape and size with application to gravel threshold. *Sedimentology* **33**, 425–436 (1986).
- 42. Schmidt, K.-H. & Ergenzinger, P. Bedload entrainment, travel lengths, step lengths, rest periods—studied with passive (iron, magnetic) and active (radio) tracer techniques. *Earth Surf. Process. Landf.* 17, 147–165 (1992).
- Schmidt, K.-H. & Gintz, D. Results of Bed load tracer experiments in a mountain river. In River Geomorphology 37–54 (Wiley, 1995).
- 44. Buffington, J. M., Dietrich, W. E. & Kirchner, J. W. Friction angle measurements on a naturally formed gravel streambed: implications for critical boundary shear stress. *Water Resour. Res.* 28, 411–425 (1992).
- 45. White, C. M. The equilibrium of grains on the bed of a stream. Proc. R. Soc. Math. Phys. Eng. Sci. 174, 322-338 (1940).
- 46. Zingg, T. Beitrag zur Schotteranalyse. (ETH Zurich, 1935).
- 47. Barrett, P. J. The shape of rock particles, a critical review. Sedimentology 27, 291–303 (1980).
- 48. Blott, S. J. & Pye, K. Particle shape: a review and new methods of characterization and classification. Sedimentology 55, 31–63 (2008).
- 49. Domokos, G., Sipos, A., Szabó, T. & Várkonyi, P. Pebbles, shapes, and equilibria. Math. Geosci. 42, 29-47 (2009).
- 50. Szabó, T. & Domokos, G. A new classification system for pebble and crystal shapes based on static equilibrium points. *Cent. Eur. Geol.* 53, 1–19 (2010).
- 51. Domokos, G., Kun, F., Sipos, A. Á. & Szabó, T. Universality of fragment shapes. Sci. Rep. 5, 9147 (2015).
- 52. Novák-Szabó, T. et al. Universal characteristics of particle shape evolution by bed-load chipping. Sci. Adv. 4, eaa04946 (2018).
- 53. Attal, M. & Lavé, J. Pebble abrasion during fluvial transport: experimental results and implications for the evolution of the sediment load along rivers. *J. Geophys. Res.* **114**, 1–22 (2009).
- 54. Auel, C., Albayrak, I., Sumi, T. & Boes, R. M. Sediment transport in high-speed flows over a fixed bed: 1. Particle dynamics. *Earth Surf. Process. Landf.* 42, 1365–1383 (2017).
- 55. Demir, T. The Influence of Particle Shape on Bedload Transport in Coarse-Bed River Channels (Durham University, Durham, 2000).
- 56. Bridge, J. S. & Bennett, S. J. A model for the entrainment and transport of sediment grains of mixed sizes, shapes, and densities. *Water Resour. Res.* **28**, 337–363 (1992).
- 57. Bagnold, R. A. An Approach to the Sediment Transport Problem from General Physics. http://pubs.er.usgs.gov/publication/pp422I (1966)
- 58. Parker, G., Klingeman, P. C. & McLean, D. G. Bedload and size distribution in paved gravel-bed streams. *ASCE J. Hydraul. Div.* **108**, 544–571 (1982).
- 59. Wilcock, P. R. & Crowe, J. C. Surface-based transport model for mixed-size sediment. J. Hydraul. Eng. 129, 120-128 (2003).
- 60. Johnston, C. E., Andrews, E. D. & Pitlick, J. In situ determination of particle friction angles of fluvial gravels. *Water Resour. Res.* 34, 2017–2030 (1998).
- 61. Sneed, E. D. & Folk, R. L. Pebbles in the lower Colorado River, Texas a study in particle morphogenesis. J. Geol. 66, 114–150 (1958).
- 62. Oakey, R. J. et al. Grain-shape analysis—a new method for determining representative particle shapes for populations of natural grains. J. Sediment. Res. 75, 1065–1073 (2005).
- 63. Cassel, M., Piégay, H. & Lavé, J. Effects of transport and insertion of radio frequency identification (RFID) transponders on resistance and shape of natural and synthetic pebbles: applications for riverine and coastal bedload tracking: transport and rfid-insertion effects on the fragility of pebbles. *Earth Surf. Process. Landf.* https://doi.org/10.1002/esp.3989 (2016).
- Cassel, M. et al. Evaluating a 2D image-based computerized approach for measuring riverine pebble roundness. Geomorphology https://doi.org/10.1016/j.geomorph.2018.03.020 (2018).
- 65. Attal, M., Lave, J. & Masson, J. P. New facility to study river abrasion processes. J. Hydraul. Eng. 132, 624-628 (2006).
- 66. Chapuis, M., Bright, C. J., Hufnagel, J. & MacVicar, B. Detection ranges and uncertainty of passive Radio Frequency Identification (RFID) transponders for sediment tracking in gravel rivers and coastal environments. Earth Surf. Process. Landf. 39, 2109–2120 (2014)
- 67. Arnaud, F., Piégay, H., Vaudor, L., Bultingaire, L. & Fantino, G. Technical specifications of low-frequency radio identification bedload tracking from field experiments: differences in antennas, tags and operators. *Geomorphology* 238, 37–46 (2015).
- Haschenburger, J. K. & Church, M. Bed material transport estimated from the virtual velocity of sediment. Earth Surf. Process. Landf. 23, 791–808 (1998).
- 69. Cecchetto, M. et al. Diffusive regimes of the motion of bed load particles in open channel flows at low transport stages. Water
 Resour Res 54, 8674–8691 (2018)
- 70. Ballio, F., Radice, A., Fathel, S. L. & Furbish, D. J. Experimental censorship of bed load particle motions and bias correction of the associated frequency distributions. *J. Geophys. Res. Earth Surf.* 124, 116–136 (2019).

Acknowledgements

Pierfrancesco Dellino is acknowledged for the thorough revision that helped to improve the manuscript. The Associate Editor, Roberto Sulpizio, and his Editorial Assistants, were very helpful and supportive during the review process.

Author contributions

M.C., H.P., and J.L. contributed to the design of the experiment. M.C., J.L., and H.P. analyzed experimental results; J.L., H.P., M.C., A.R., and J-R.M. wrote the manuscript.

Competing interests

The authors declare no competing interests.

Additional information

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